

DYNAMICS OF STRUCTURES

Single Degree of Freedom System: Free Vibration Response

Free Vibration: Viscous Damping

- The free vibration response an damped (viscous) system is governed by:

$$m\ddot{u} + c\dot{u} + ku = 0$$

- A possible solution of the above equation is:

$$u = Ge^{\lambda t}$$

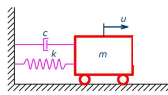
- Substituting the solution into equation of motion:

$$G\lambda^2 me^{\lambda t} + c\lambda e^{\lambda t} + Gke^{\lambda t} = 0$$

$$\lambda^2 m + c\lambda + k = 0$$

$$\Rightarrow \lambda^2 + \frac{c}{m}\lambda + \frac{k}{m} = 0$$

$$\lambda = -\frac{c}{2m} \pm \sqrt{\frac{c^2}{4m^2} - \frac{k}{m}}$$



Free Vibration: Viscous Damping (Cont..)

- Based on the value of the discriminant, three different cases arise:

$$\lambda = -\frac{c}{2m} \pm \sqrt{\frac{c^2}{4m^2} - \frac{k}{m}}$$

- Critically damped system (discriminant is zero) $\frac{c^2}{4m^2} - \frac{k}{m} = 0 \Rightarrow c = c_c = 2\sqrt{km} = 2m\omega$
- Overdamped system (discriminant is positive) $\frac{c^2}{4m^2} - \frac{k}{m} > 0 \Rightarrow c > (c_c = 2m\omega) \Rightarrow \left(\zeta = \frac{c}{c_c}\right) > 1$
- Underdamped system (discriminant is negative) $\frac{c^2}{4m^2} - \frac{k}{m} < 0 \Rightarrow c < (c_c = 2m\omega) \Rightarrow \left(\zeta = \frac{c}{c_c}\right) < 1$

Critically Damped System

- For critically damped system

$$\frac{c^2}{4m^2} - \frac{k}{m} = 0 \Rightarrow c = c_c = 2\sqrt{km} = 2m\omega$$

The roots of the differential equation are:

$$\lambda_1 = \lambda_2 = -\frac{c}{2m} = -\frac{c_c}{2m} = -\omega$$

Lambda is equal to omega on for critically damped system

Therefore the solution of equation of motion is:

$$u = (G_1 + G_2 t) e^{-\omega t}$$

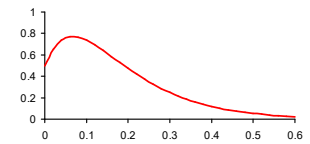
The arbitrary constants are determined from the initial condition. The general solution thus becomes:

$$u = \{u_0 + (v_0 + \omega u_0) t\} e^{-\omega t}$$

For critically damped

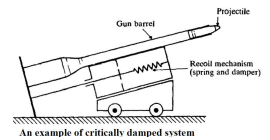
Critically Damped System (Cont..)

- Thus the free vibration of a critically damped system is **not oscillatory**



- After the initial disturbance the system will **come back to its original position** without oscillation in maximum time.

- Examples:
Recoiling Gun and Weighing Scale



Overdamped System

- For overdamped system, damping is greater than critical damping
- The ratio of damping of a system to its critical damping is called damping ratio and is given by:

$$\zeta = \frac{c}{c_c} = \frac{c}{2m\omega} \Rightarrow c = 2m\omega\zeta$$

- The roots of characteristic equation may be written as:

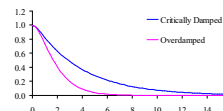
$$\lambda = -\omega\zeta \pm \omega\sqrt{\zeta^2 - 1} = -\omega\zeta \pm \bar{\omega}$$

$$\text{where } \bar{\omega} = \omega\sqrt{\zeta^2 - 1}$$

- The general solution of equation of motion is of the form:

$$u = e^{-\omega\zeta t} (A \cosh \bar{\omega} t + B \sinh \bar{\omega} t)$$

Examples are: Automatic door closer



Underdamped System

- For underdamped system, damping is less than the critical damping, i.e. the damping ratio is less than one
- The roots of characteristic equation may be written as:

$$\lambda = -\omega\zeta \pm i\omega\sqrt{1 - \zeta^2} = -\omega\zeta \pm \omega_d$$

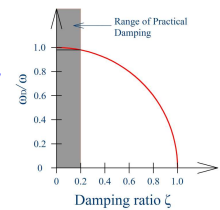
$$\text{where } \omega_d = \omega\sqrt{1 - \zeta^2}$$

ω_d is known as the **damped circular frequency**

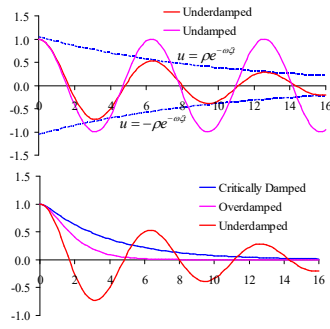
- The general solution of equation of motion is of the form:

$$u = e^{-\omega\zeta t} (A \cos \omega_d t + B \sin \omega_d t)$$

$$u = e^{-\omega\zeta t} \left(u_0 \cos \omega_d t + \frac{v_0 + \omega\zeta u_0}{\omega_d} \sin \omega_d t \right)$$



Underdamped System (Cont..)

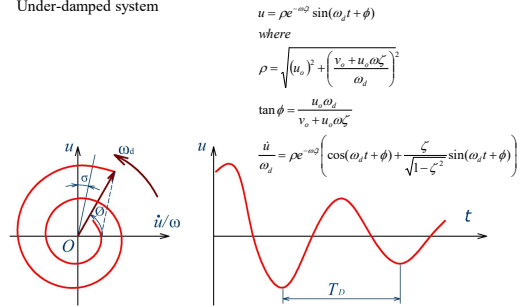


Dynamics of Structures

9

Phase Plane Diagram

- Under-damped system



Dynamics of Structures

10

$$\frac{e^{-\omega \xi t_1}}{e^{-\omega \xi (t_1 + \frac{2\pi}{\omega_d})}} = \frac{e^{-\omega \xi t_1}}{e^{-\omega \xi t_1} \times e^{-\omega \xi \frac{2\pi}{\omega_d}}} = e^{\omega \xi \frac{2\pi}{\omega_d}}$$

$$\delta = \ln(e^{\omega \xi \frac{2\pi}{\omega_d}}) = \omega \xi \frac{2\pi}{\omega_d} = 2\pi \xi \frac{\omega}{\omega_d} = \frac{2\pi \xi}{\sqrt{1-\xi^2}}$$

Logarithmic Decrement

- It is the natural log of the ratio of displacement at any time t_1 and $t_1 + 2\pi/\omega_d$
- It represent decay in the magnitude during one cycles
- Logarithmic Decrement is used to calculate the damping ratio of a system

$$u(t_1) = \rho e^{-\omega \xi t_1} \sin(\omega_d t_1 + \phi)$$

$$u\left(t_1 + \frac{2\pi}{\omega_d}\right) = \rho e^{-\omega \xi \left(t_1 + \frac{2\pi}{\omega_d}\right)} \sin\left(\omega_d \left(t_1 + \frac{2\pi}{\omega_d}\right) + \phi\right) = \rho e^{-\omega \xi \left(t_1 + \frac{2\pi}{\omega_d}\right)} \sin(\omega_d t_1 + \phi)$$

$$\delta = \ln\left(\frac{u(t_1)}{u\left(t_1 + \frac{2\pi}{\omega_d}\right)}\right) = \ln\left(\frac{e^{-\omega \xi t_1}}{e^{-\omega \xi \left(t_1 + \frac{2\pi}{\omega_d}\right)}}\right) = \ln\left(\frac{e^{-\omega \xi t_1}}{e^{-\omega \xi t_1} \times e^{-\omega \xi \frac{2\pi}{\omega_d}}}\right) = \ln\left(e^{\omega \xi \frac{2\pi}{\omega_d}}\right)$$

$$\delta = \frac{2\pi \xi}{\sqrt{1-\xi^2}} \quad \delta = \ln(e^{\omega \xi \frac{2\pi}{\omega_d}}) = \omega \xi \frac{2\pi}{\omega_d} = 2\pi \xi \frac{\omega}{\omega_d} = \frac{2\pi \xi}{\sqrt{1-\xi^2}}$$

$$\zeta = \frac{\delta}{\sqrt{4\pi^2 + \delta^2}}$$

Dynamics of Structures

11

Logarithmic Decrement (Cont..)

- For many practical cases:
 $\zeta < 0.2$ therefore $1 - \zeta^2 \approx 1$
 $\delta = 2\pi \zeta \Rightarrow \zeta = \frac{\delta}{2\pi}$
- For lightly damped system, the decay of motion is slow, it is desirable to relate amplitudes several cycles apart:

$$\frac{u_1}{u_{N+1}} = \frac{u_1}{u_2} \frac{u_2}{u_3} \frac{u_3}{u_4} \dots \frac{u_N}{u_{N+1}} = e^{N\delta}$$

$$\ln \frac{u_1}{u_{N+1}} = N\delta = N2\pi\zeta$$

$$\zeta = \frac{1}{2\pi N} \ln \frac{u_1}{u_{N+1}}$$

Dynamics of Structures

12

Logarithmic Decrement (Cont..)

- The number of cycles in which the amplitude will decay to half its value at the beginning is given by:

$$N_{50\%} = \frac{1}{\delta} \ln 2 \approx \frac{1}{2\pi\zeta} \ln 2 \approx \frac{0.11}{\zeta}$$

- Since acceleration is easy to measure and also acceleration is proportional to the displacement in case of free vibration, the damping ratio is therefore determined from:

$$\zeta = \frac{1}{2\pi N} \ln \frac{\ddot{u}_1}{\ddot{u}_{N+1}}$$

